



# Natural spring' protection and groundwater management under uncertainty conditions: the Cremona Aquifer



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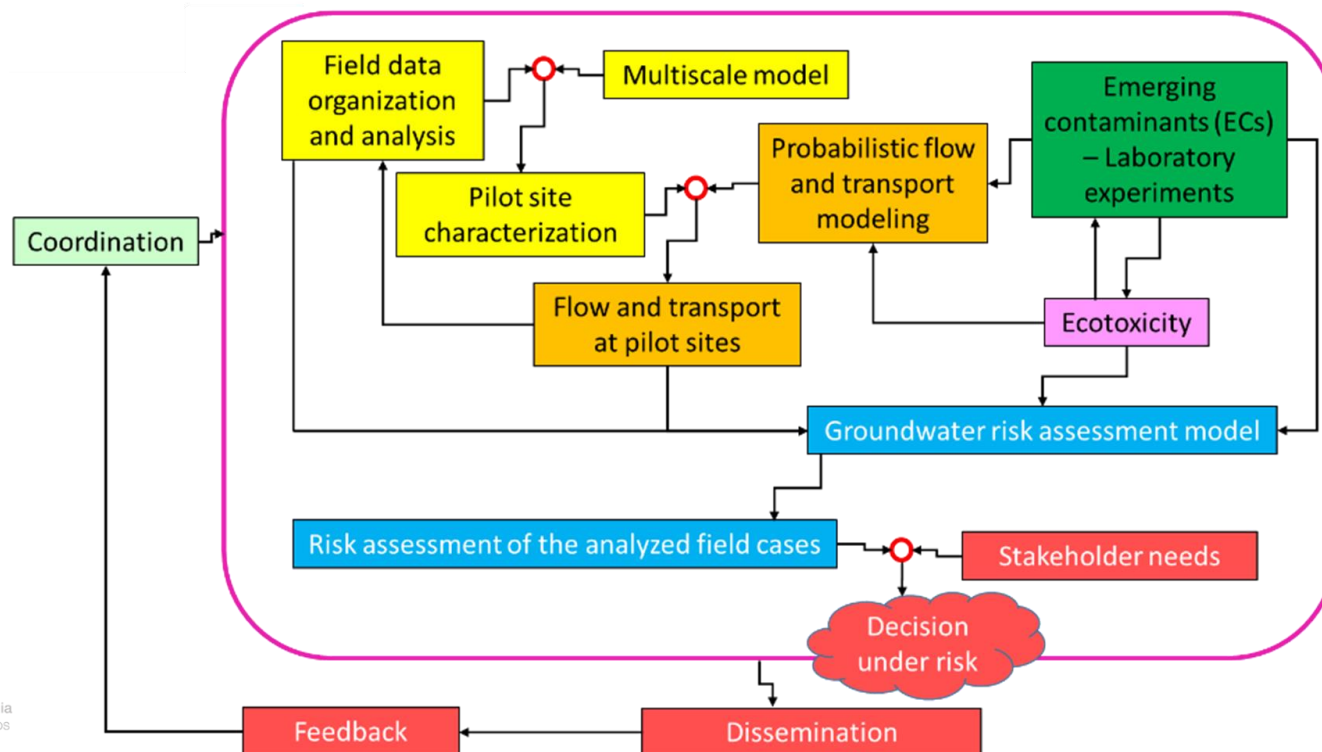
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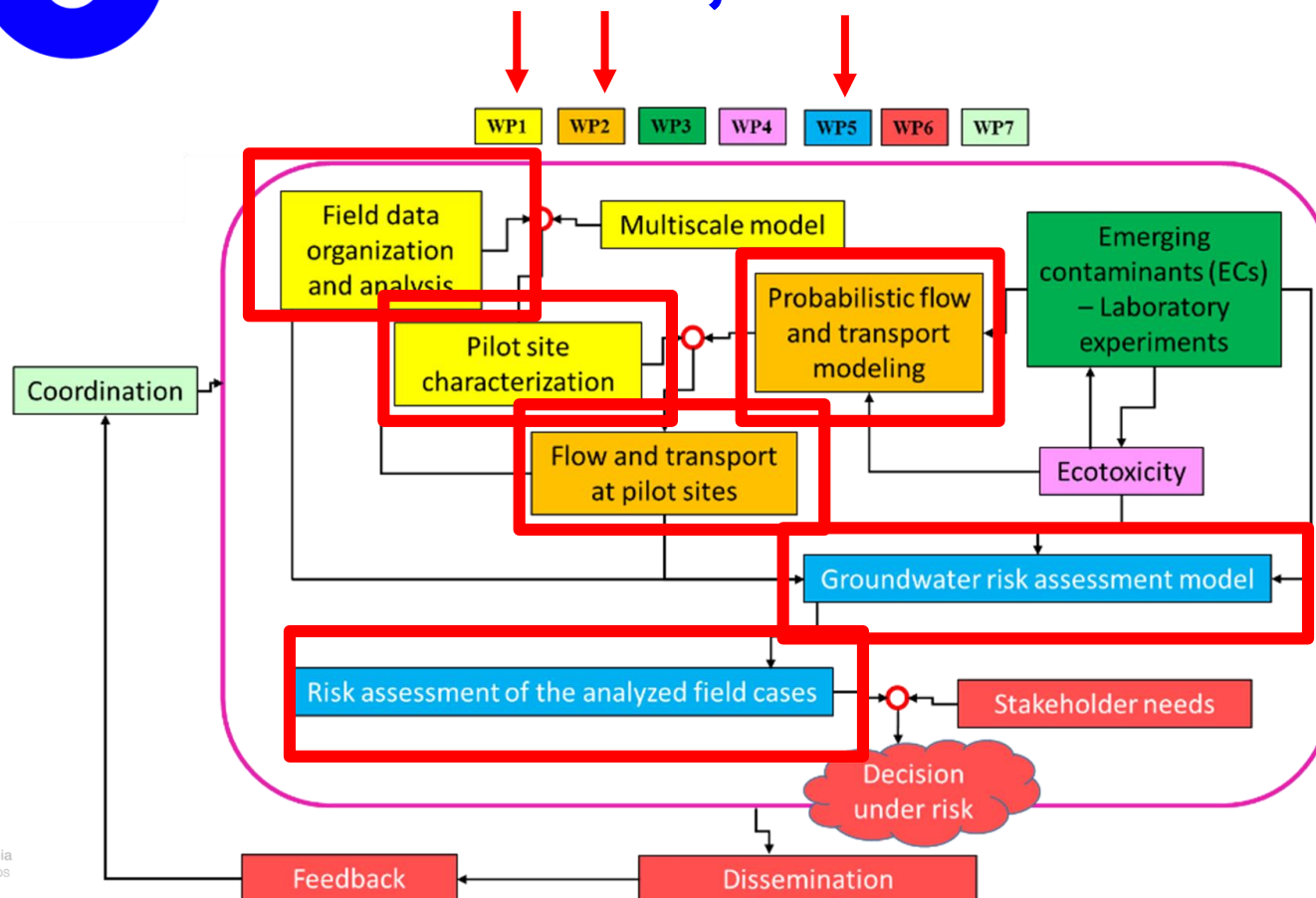
# WE-NEED Project Structure



Consorzio di bonifica  
Dugli  
Naviglio  
Adda Serio



# WE-NEED Project Structure



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Consorzio di bonifica  
Dugli Naviglio  
Adda Serio

# Outline

## Test case description and model conceptualizations

- Study area and available data set
- Spatial distribution of geomaterials
- Hydraulic conductivity fields

WP1

## Global sensitivity and model calibration approaches

- Derivative-based, Variance-based, Moment-based sensitivity indices
- Model calibration

WP1

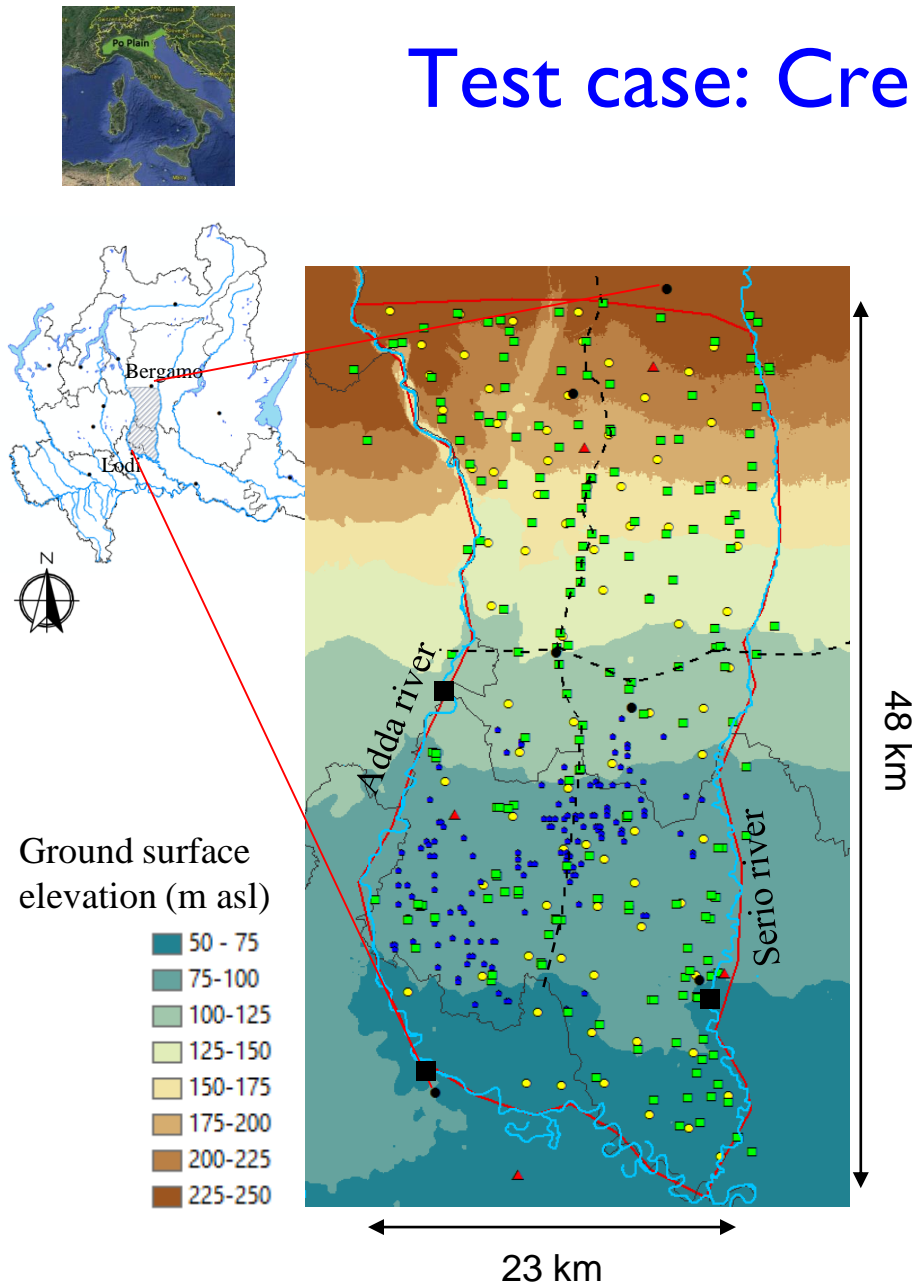
WP2

## Groundwater management model

- Spring depletion problem
- Fault tree analysis and evaluation of probability of system failure

WP5

# Test case: Cremona Aquifer

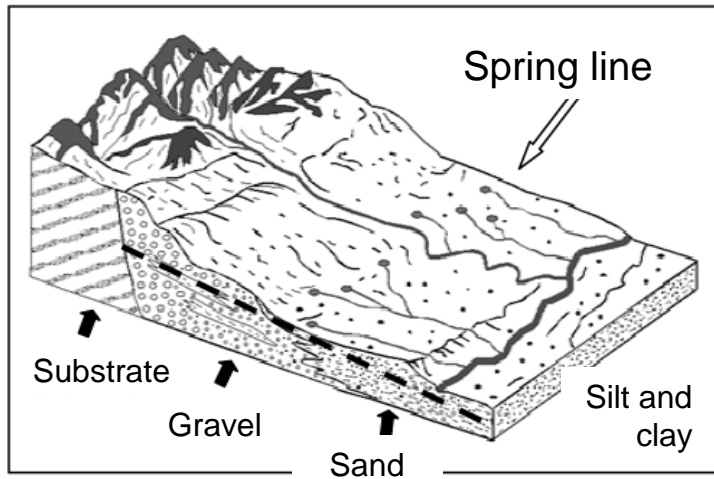


- The study area lies in the Lombardia region (Northern Italy)
- Adda and Serio rivers bound the aquifer on eastern, southern and western sides
- Surface Area: 785 km<sup>2</sup> (84% agricultural)
- A key feature of the study area is the occurrence of natural high-quality water springs

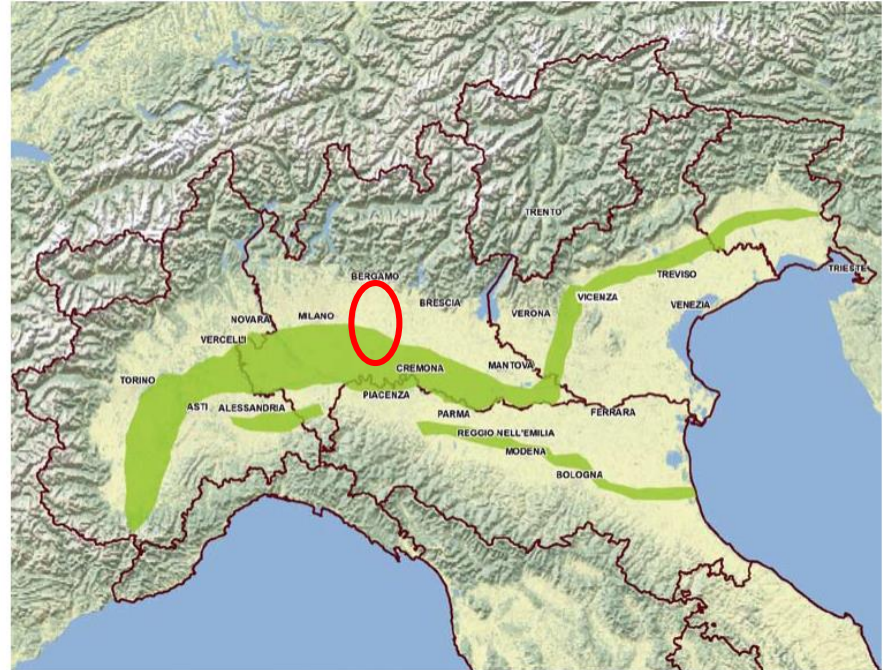
- Hydrometric level station
- ▲ Meteorological station
- Well
- Geological stratigraphy
- Spring



# “Fontanili” a particular kind of spring in the Po Plain



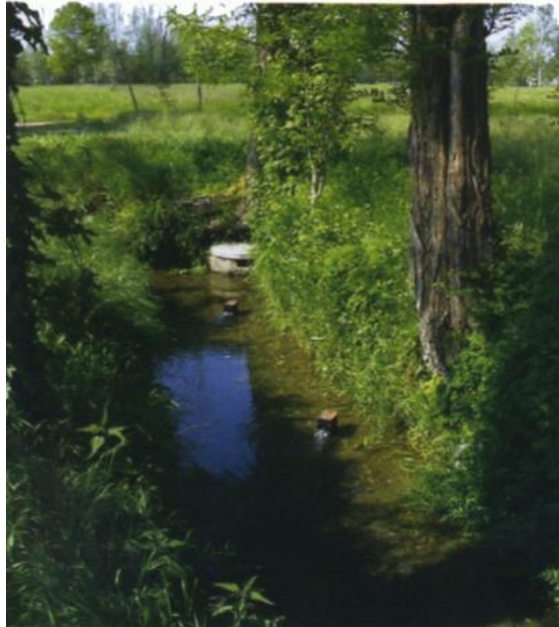
Illustrative draft of the Po Plain and types of prevalent deposits (Giornale di Geologia Applicata 2 (2005) 377–382)



“Fontanili Line” (QdR n. 144, 2012 Regione Lombardia)

- “Fontanili Line” is characterized by spring subsurface water emergence, produced by a sediment permeability decrease.
- A great number of “Fontanili” consists of an excavation that reaches the unconfined aquifer.

## “Fontanili” within the study area



Brunascani (CR)



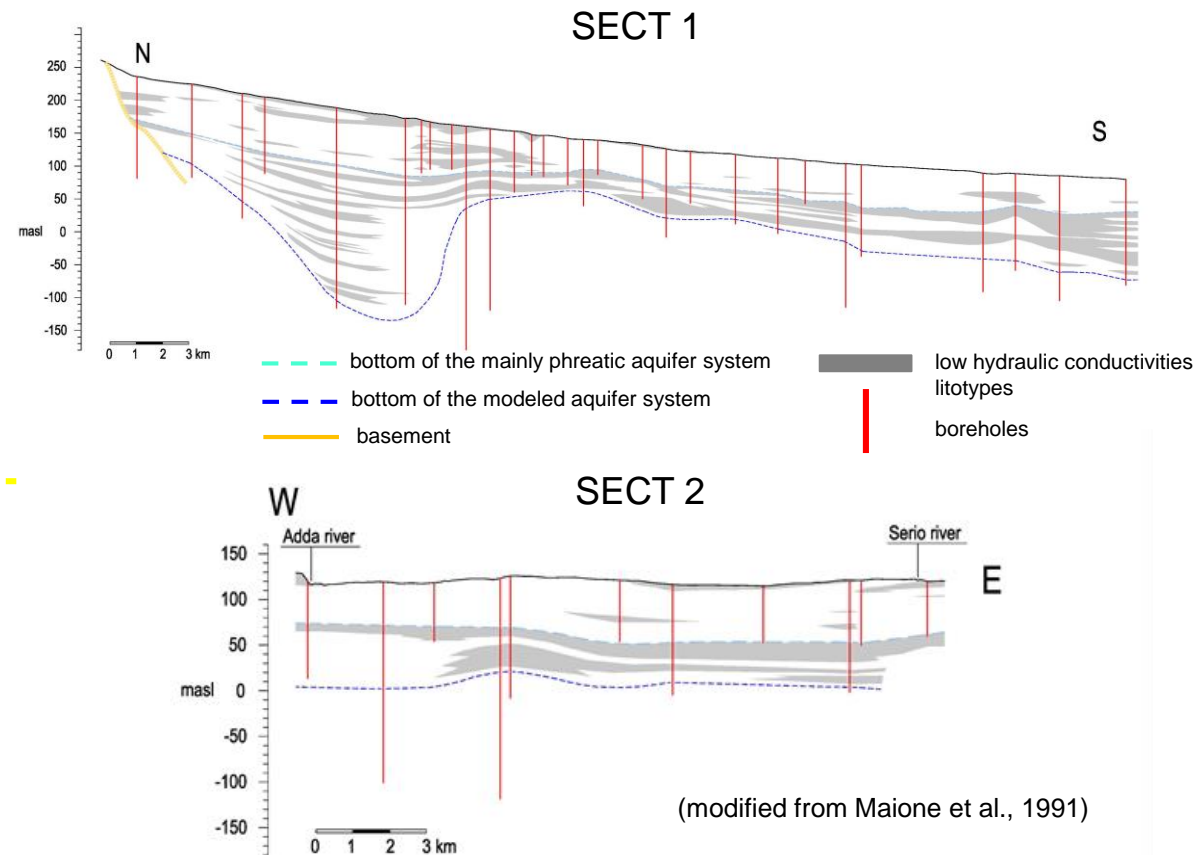
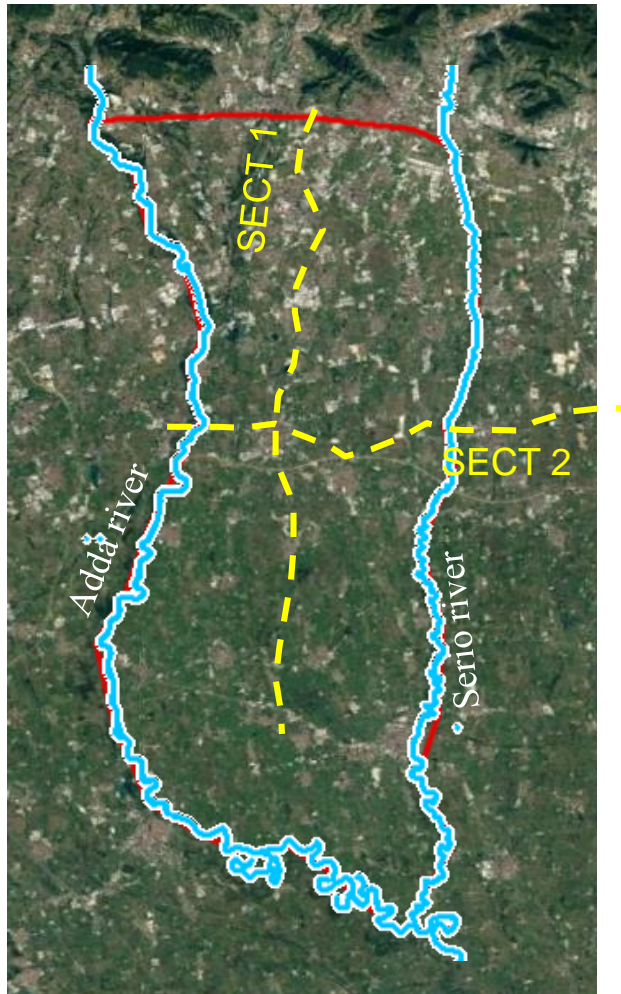
Cremosano Est (CR)



Cascinetto (CR)



# Geological cross-sections








The aquifer system has an average thickness of about 120 m and comprises:

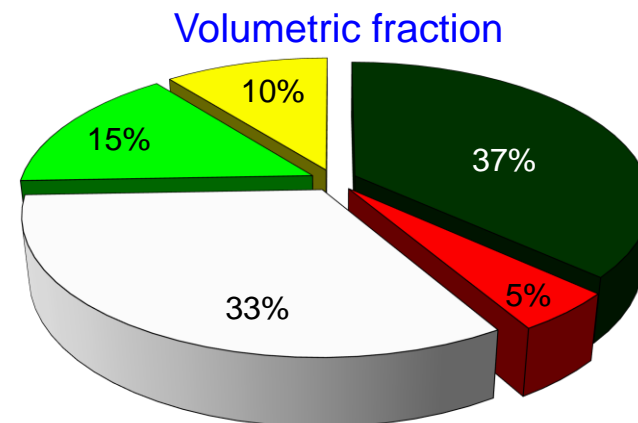
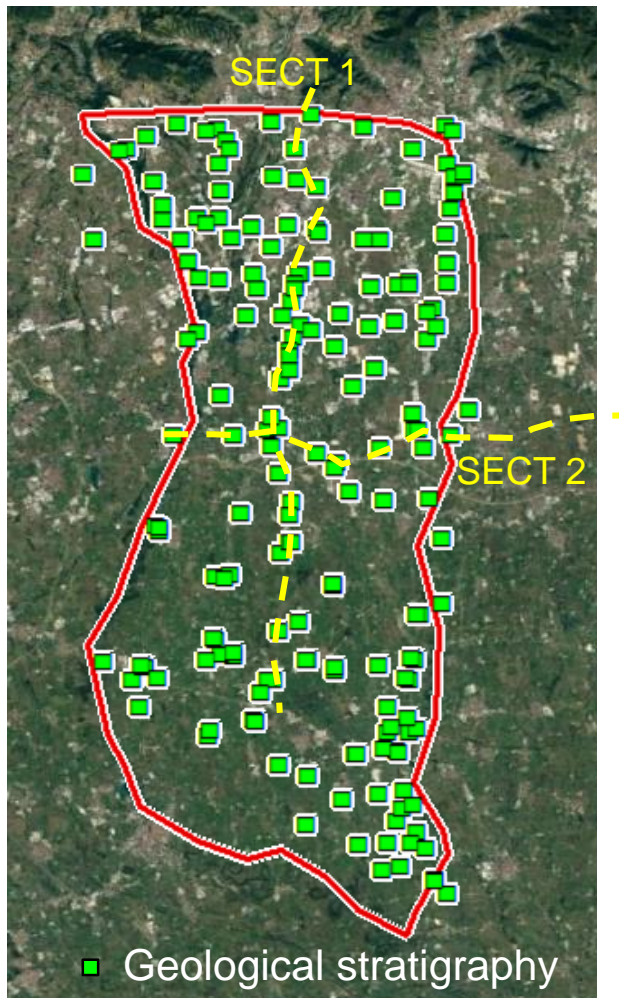
- a surface, locally semiconfined, aquifer
- a deeper, semiconfined/confined portion



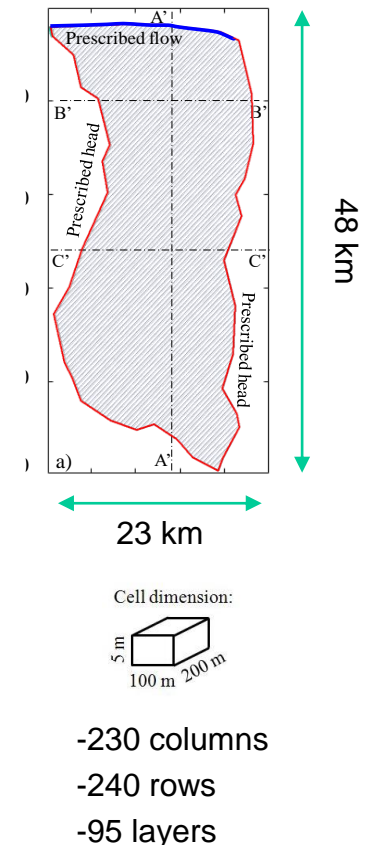
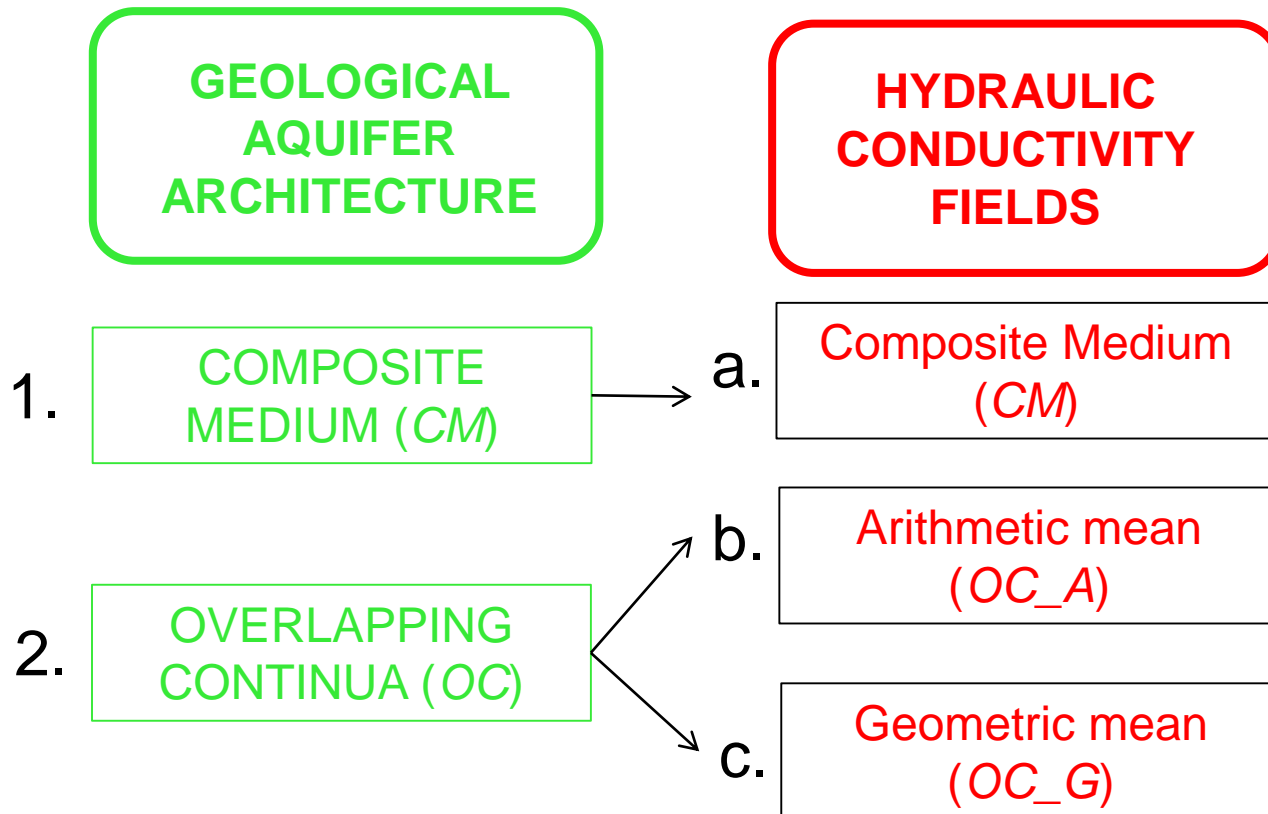
# Geological stratigraphies

- The analysis of available sedimentological information allows identifying a set of  $n_f = 5$  main geomaterials (facies/classes) which constitute the geological makeup of the system.

FACIES	GEOLOGICAL MATERIAL
1 	Clay and silty deposits
2 	Fine Sands, Clay Sands, Silty Sands
3 	Gravel, Gravel and Sand, Medium Sand
4 	Compact Conglomerates
5 	Fractured Conglomerates



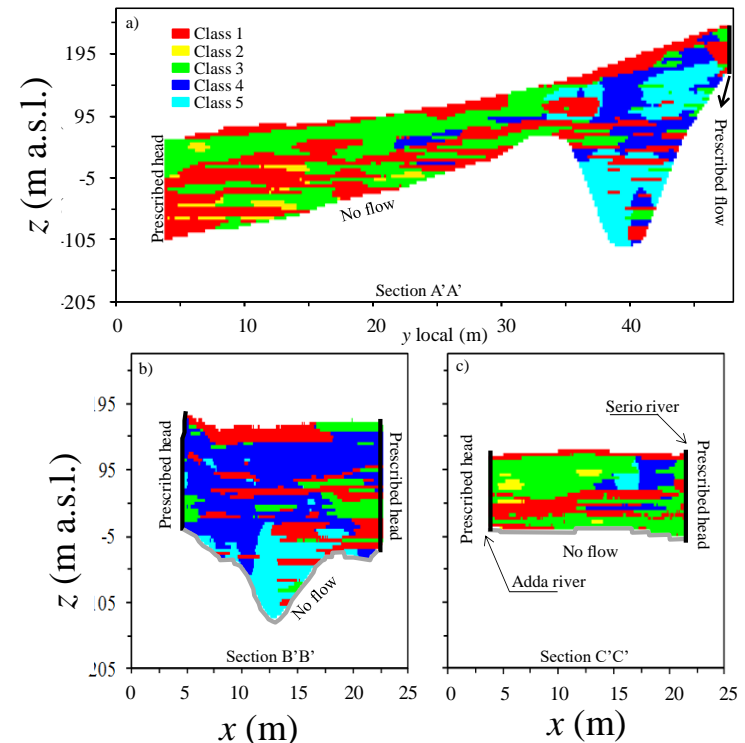
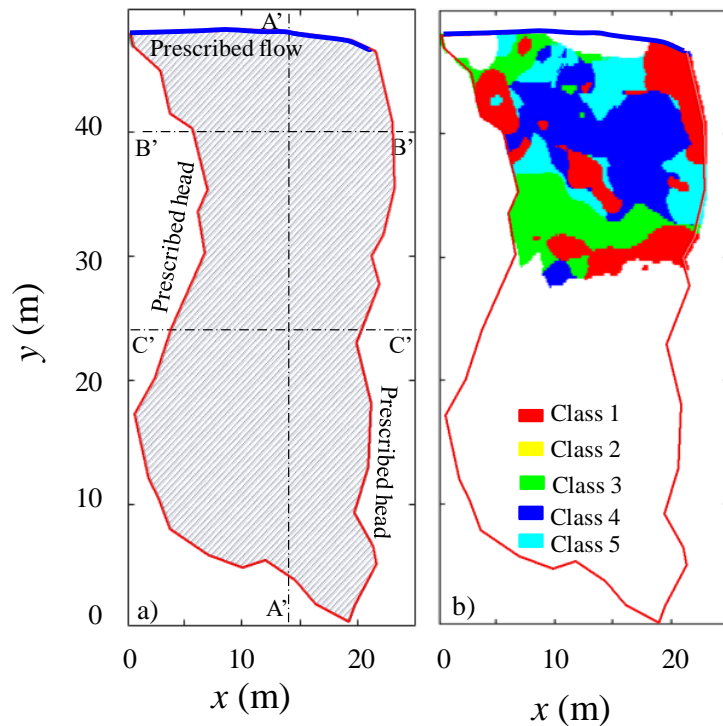
- We discretize the aquifer system of extent 23 km (East-West direction) × 48 km (North-south direction) × 475 m (depth) through blocks of uniform size 100 m × 200 m × 5 m. Total of  $N_c = 5.2 \times 10^6$  voxels
- Numerical code MODFLOW-2005 (Harbaugh, 2005) is employed to simulate steady-state groundwater flow within the domain



# Composite medium (CM) approach

- Each block of the numerical model is formed by a single geomaterials (e.g., Guadagnini et al., 2004).
- The conductivity value assigned to each cell of the model domain consists in one (constant) value for each facies.

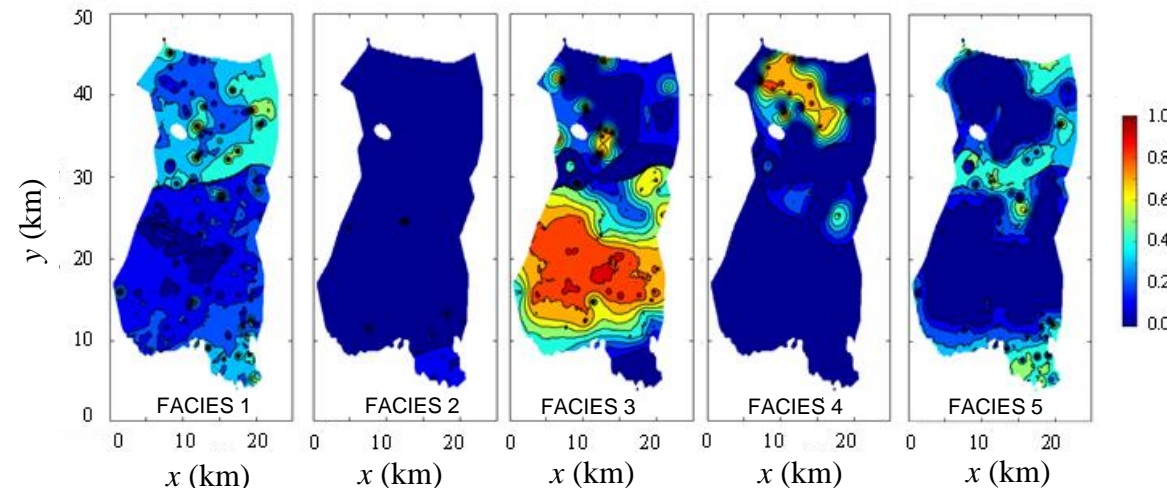
$$K_j^{CM} = k_i$$



# Overlapping continua (OC) approach

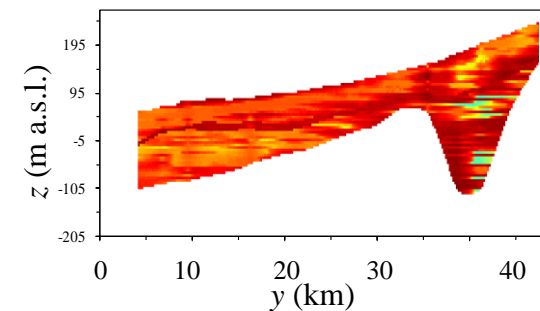
- Each voxel  $j$  of the numerical grid represents a finite volume in which all geomaterials (or hydro-facies) can coexist, each associated with a given volumetric fraction
- Conditional Indicator Kriging yields  $n_f \times N_c$  values of the Indicator function corresponding to the estimated probability that a given geomaterials (class  $M_i$ ) resides within block  $j$

Indicator function



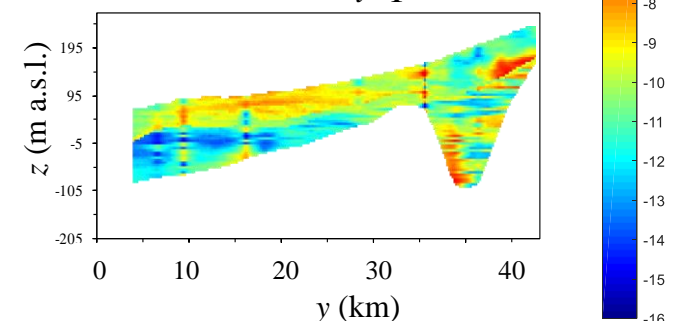
Arithmetic mean (OC\_A)

$$K_j^{OC-A} = \sum_{i=1}^{n_f} I_{i,j} k_i$$



Geometric mean (OC\_G)

$$K_i^{OC-G} = \prod_{i=1}^{n_f} k_i^{I_{i,j}}$$



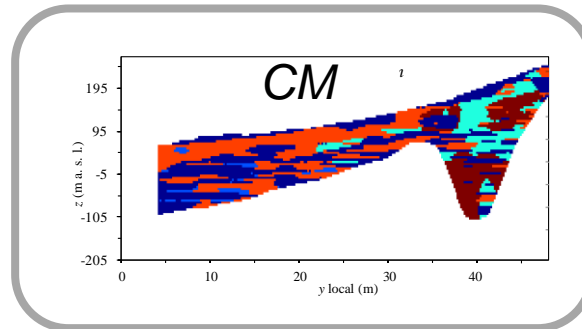


# GEOLOGICAL AQUIFER ARCHITECTURE

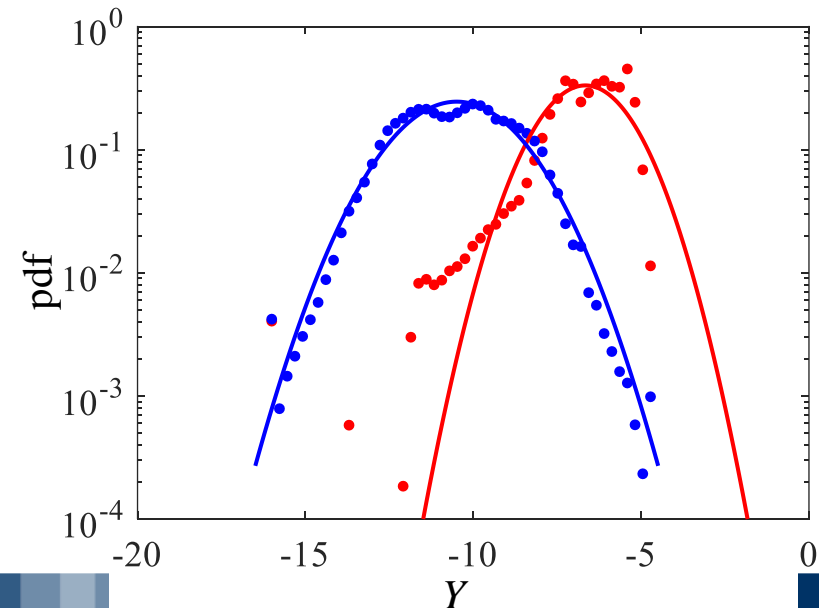
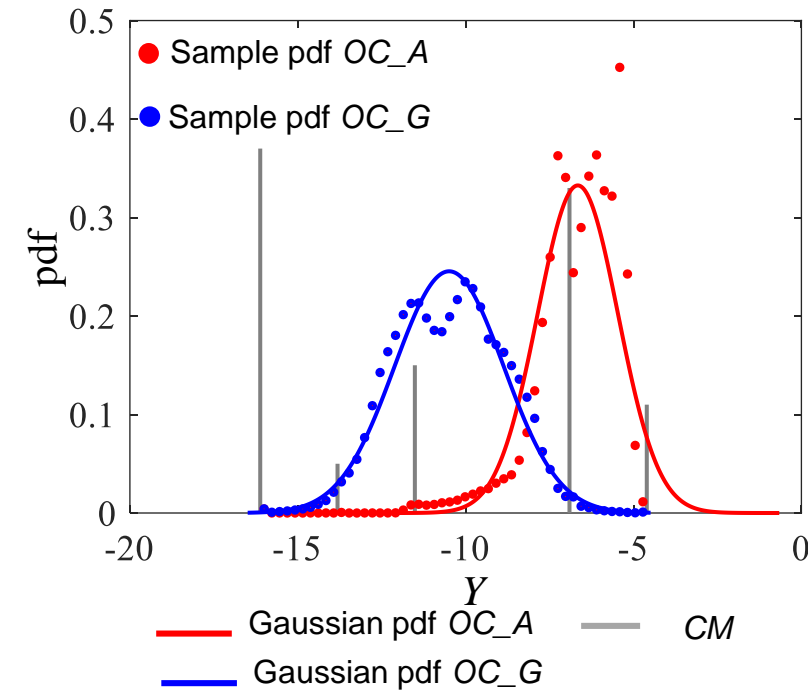
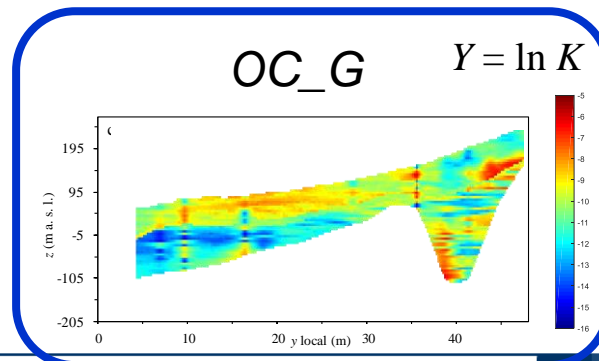
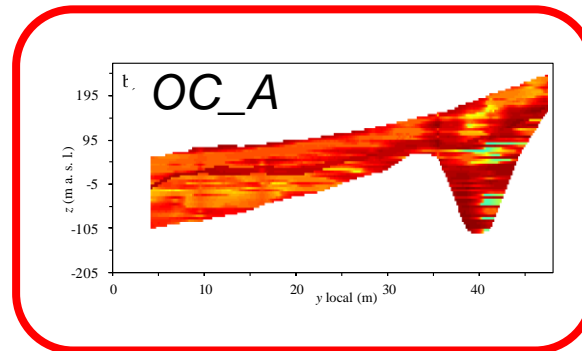
# HYDRAULIC LOG- CONDUCTIVITY FIELDS

13

COMPOSITE  
MEDIUM (CM)



OVERLAPPING  
CONTINUA (OC)

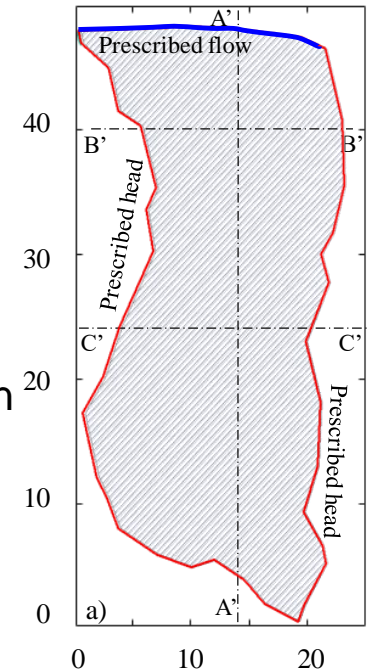


# Global sensitivity analysis

- We analyse the impact on the groundwater system response of the uncertainty in
  - conceptual model (OC\_A, OC\_G, CM)
  - boundary conditions
  - hydraulic parameters (hydraulic conductivities of the 5 facies)
- Width of the intervals associated with parameter variability is based on geological features

## 7 sources of uncertainty:

Parameter	Short name	Description	Lower bound	Upper bound	Unit
$p_1$	$k_1$	Clay and silt conductivity	$10^{-8}$	$10^{-5}$	m/s
$p_2$	$k_2$	Fine and silty sand conductivity	$10^{-7}$	$10^{-4}$	m/s
$p_3$	$k_3$	Gravel, sand and gravel conductivity	$10^{-4}$	$10^{-2}$	m/s
$p_4$	$k_4$	Compact conglomerate conductivity	$10^{-6}$	$10^{-3}$	m/s
$p_5$	$k_5$	Fractured conglomerate conductivity	$10^{-3}$	$10^{-1}$	m/s
$p_6$	$p_6$	Total flow rate from northern boundary	4.83	14.47	m <sup>3</sup> /s
$p_7$	$p_7$	Height of the river	0.0	3.0	m



Facies  
conductivity  
values

Boundary  
conditions

$$y = f(\mathbf{p})$$

# Sensitivity metrics

## Methodology

## Sensitivity

## Definition

### 1. Derivative-based Morris Indices

Local derivatives of the model output with respect to input parameters

$$EE_{p_i}(j) = \frac{f(p_1, \dots, p_i + \Delta, \dots, p_N) - f(\mathbf{p})}{\Delta}$$

$$\mu_{p_i}^* = \frac{1}{r} \sum_{j=1}^r |EE_i(j)|$$

### 2. Variance-based Sobol' indices

Expect **reduction** of the Variance of  $f$  due to the knowledge of  $p_i$

$$S_{p_i} = \frac{E[V_f - V[f|p_i]]}{V_f}$$

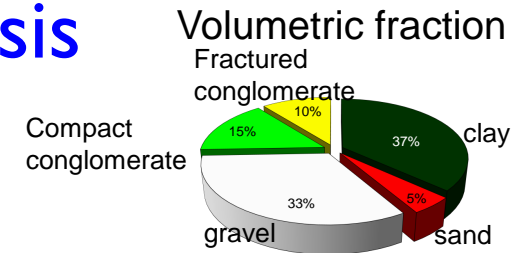
### 3. Moment-based AMAM Indices

Expected variation of Mean, Variance and Skewness of  $f$  to the knowledge of  $p_i$

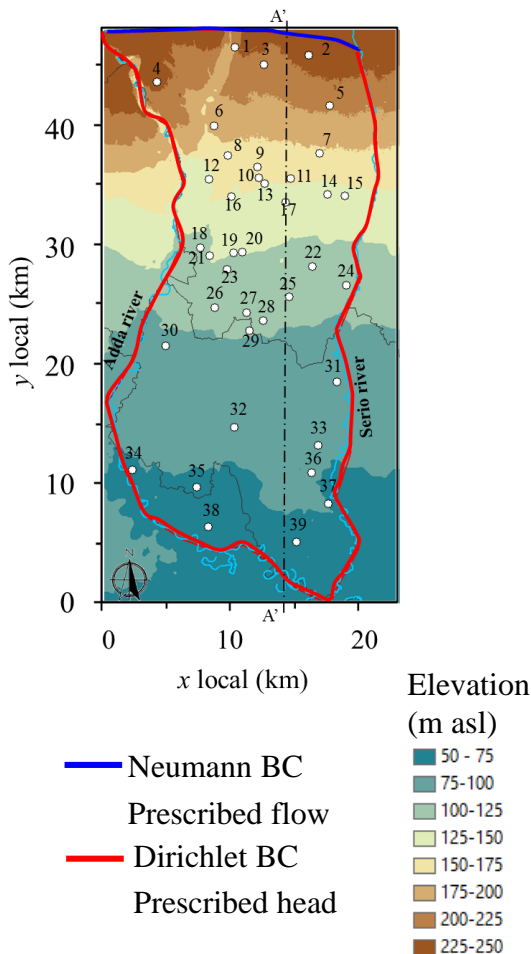
$$AMAM_{p_i} = \frac{E[|M_f - M[f|p_i]|]}{|M_f|}$$

# Global sensitivity analysis

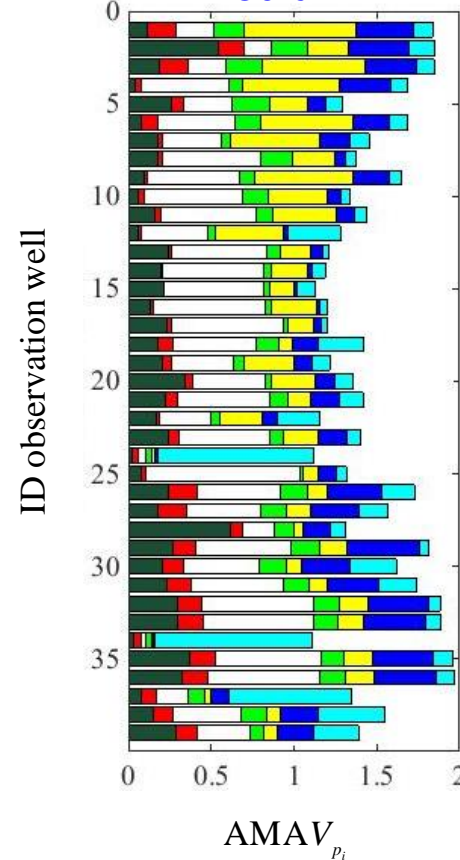
$$AMAV_{p_i} = \frac{E \left[ \left| V_f - V[f|p_i] \right| \right]}{|V_f|}$$



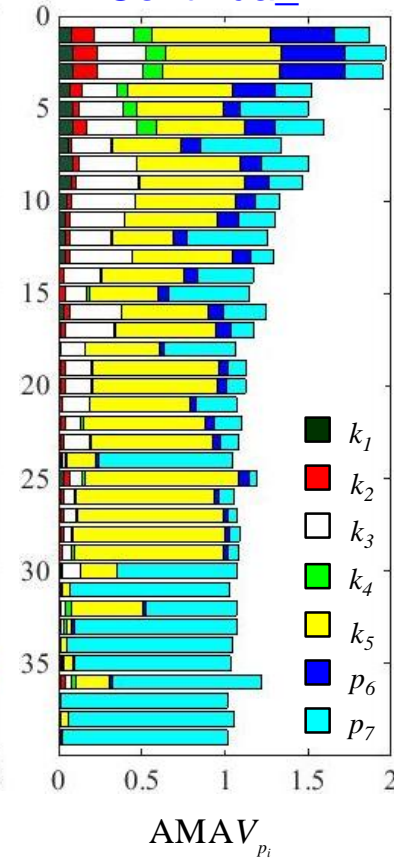
Location of observation wells



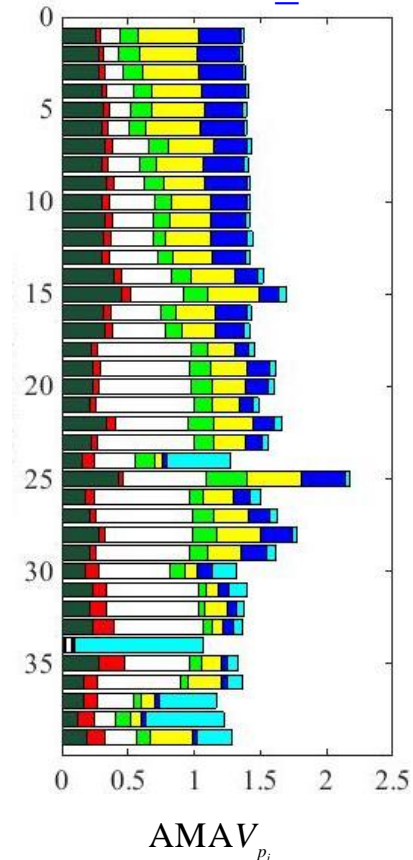
Composite Medium



Overlapping Continua\_A



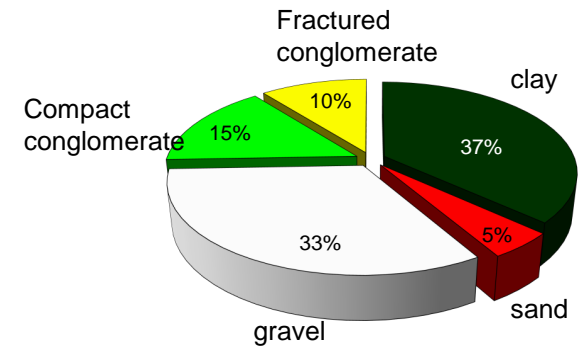
Overlapping Continua\_G





# Average of sensitivity indices

Volumetric fraction



Facies conductivity values

- $k_1$
- $k_2$
- $k_3$
- $k_4$
- $k_5$

Boundary conditions

- $p_6$
- $p_7$

$$\bar{\mu}_{p_i}^*$$

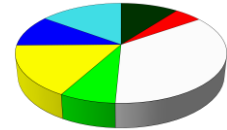
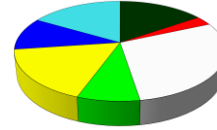
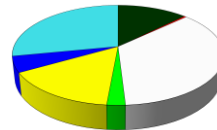
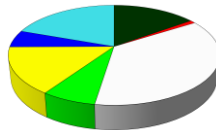
$$S_{p_i}^T$$

$$AMAE_{p_i}$$

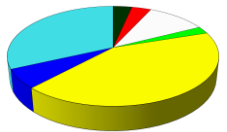
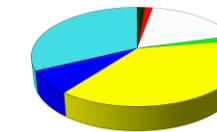
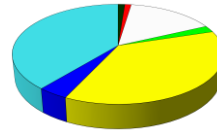
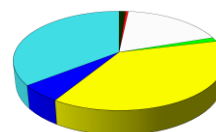
$$AMAV_{p_i}$$

$$AMA\gamma_{p_i}$$

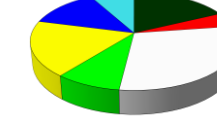
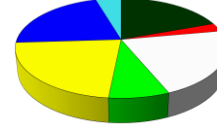
Composite Medium



Overlapping Continua\_A



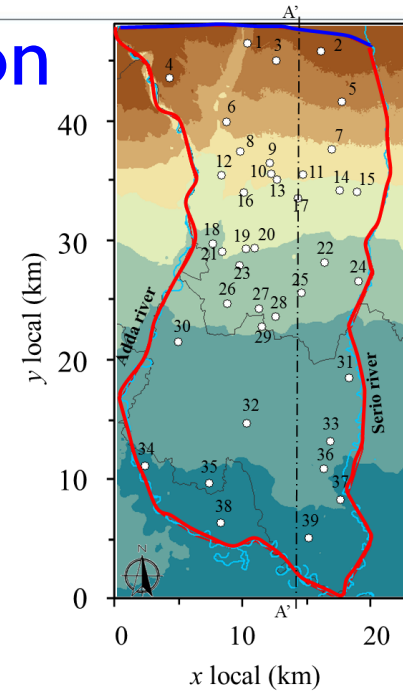
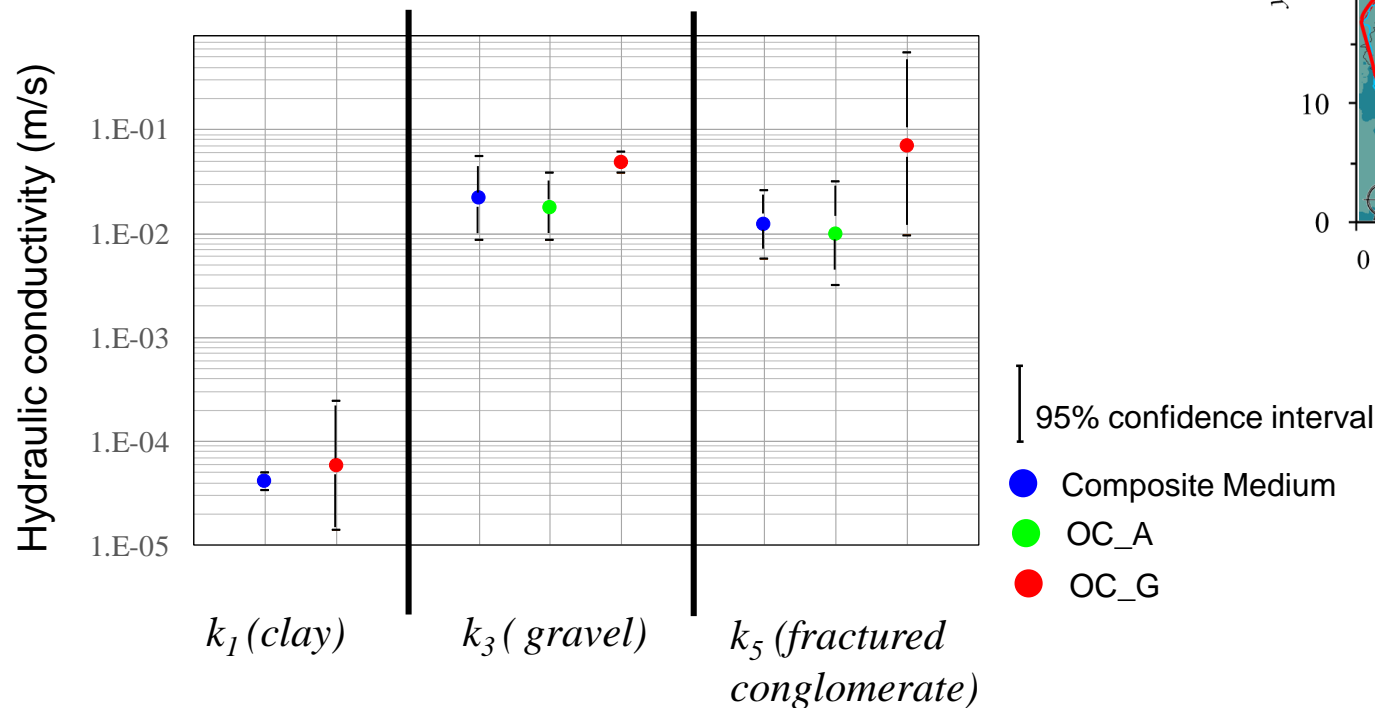
Overlapping Continua\_G



# Hydraulic conductivity calibration

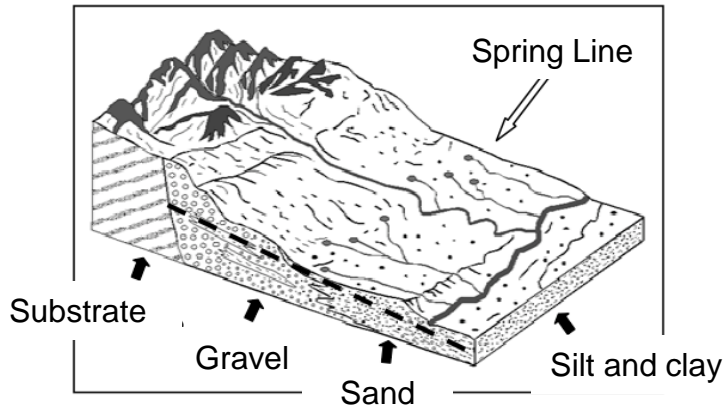
- Each selected model have been calibrated within a Maximum Likelihood approach

$$NLL = \sum_i^{N_h} \frac{J_i}{\sigma_h^2} + \ln |\mathbf{C}_h| + N_h \ln(2\pi) \quad J_i = (h_i^* - h_i)^2$$

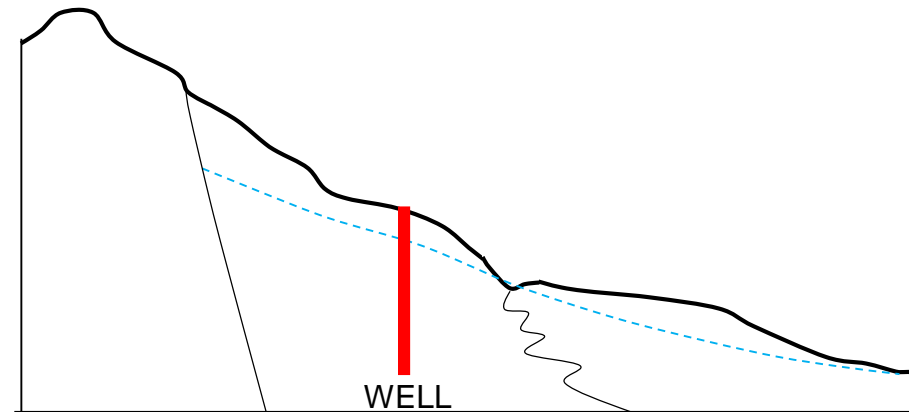
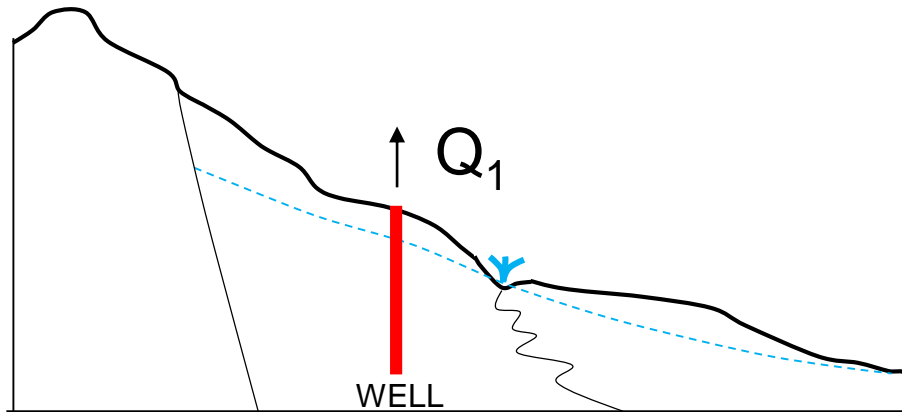


- The estimated values are consistent with the geological features of the classes
- The estimated values are consistent with the selected modeling approach

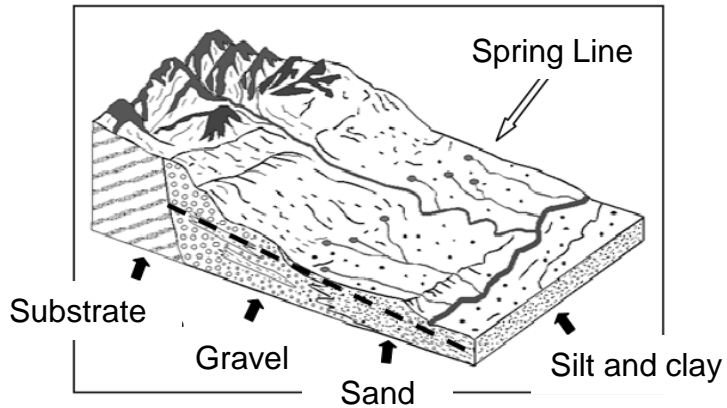
# Spring depletion problem



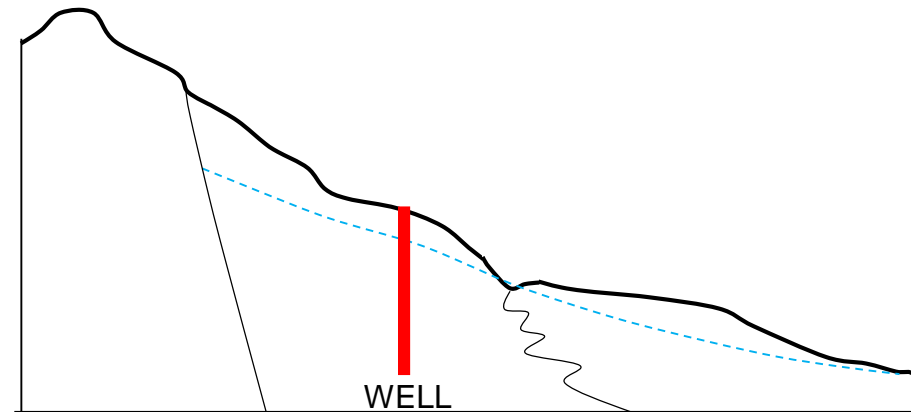
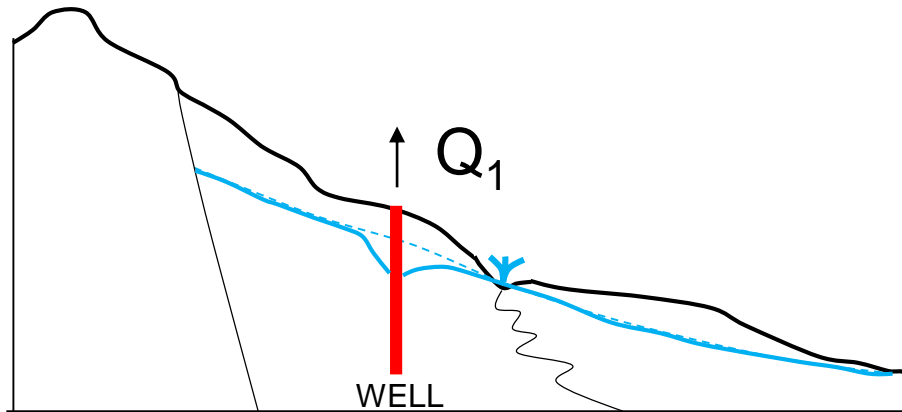
- What is the probability of spring depletion due to increasing exploitation of the aquifer?



# Spring depletion problem

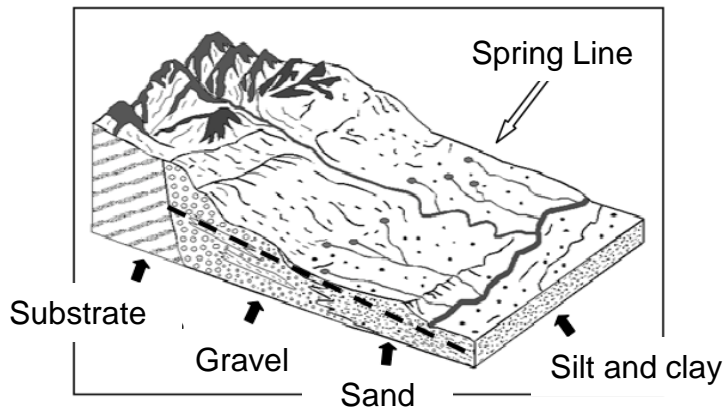


- What is the probability of spring depletion due to increasing exploitation of the aquifer?

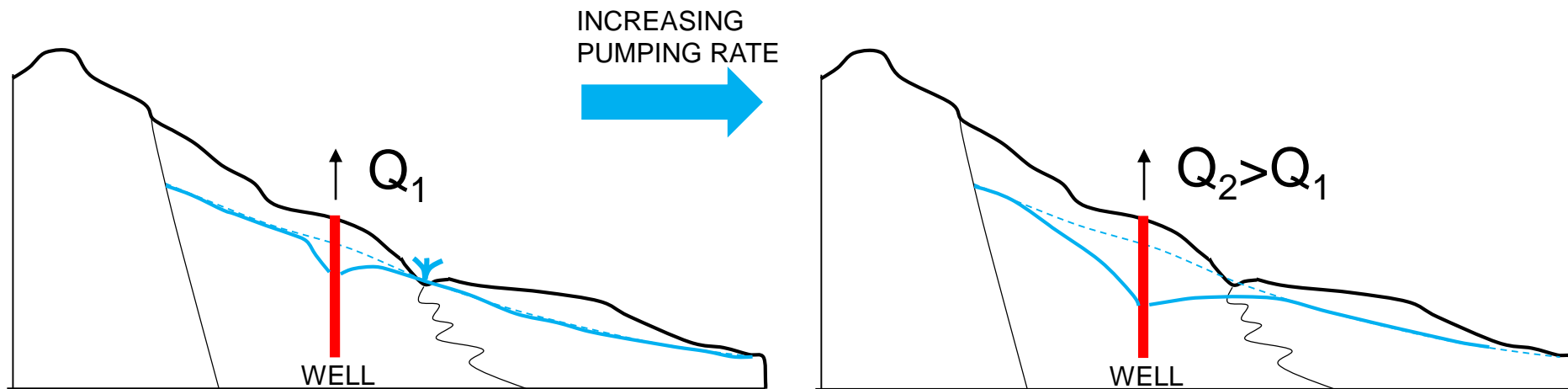




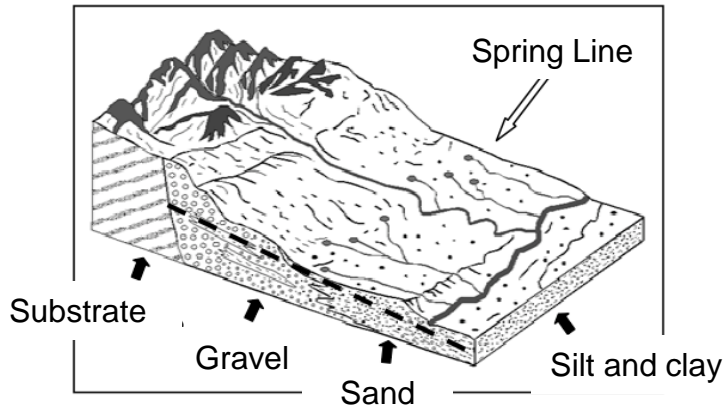
# Spring depletion problem



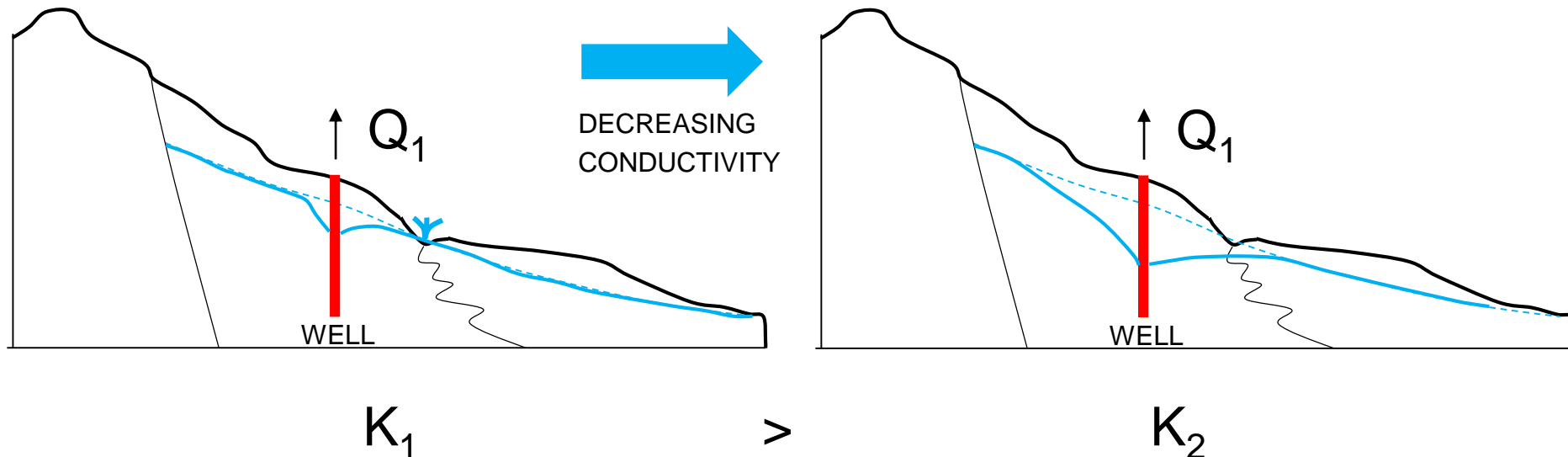
- What is the probability of spring depletion due to increasing exploitation of the aquifer?



# Spring depletion problem

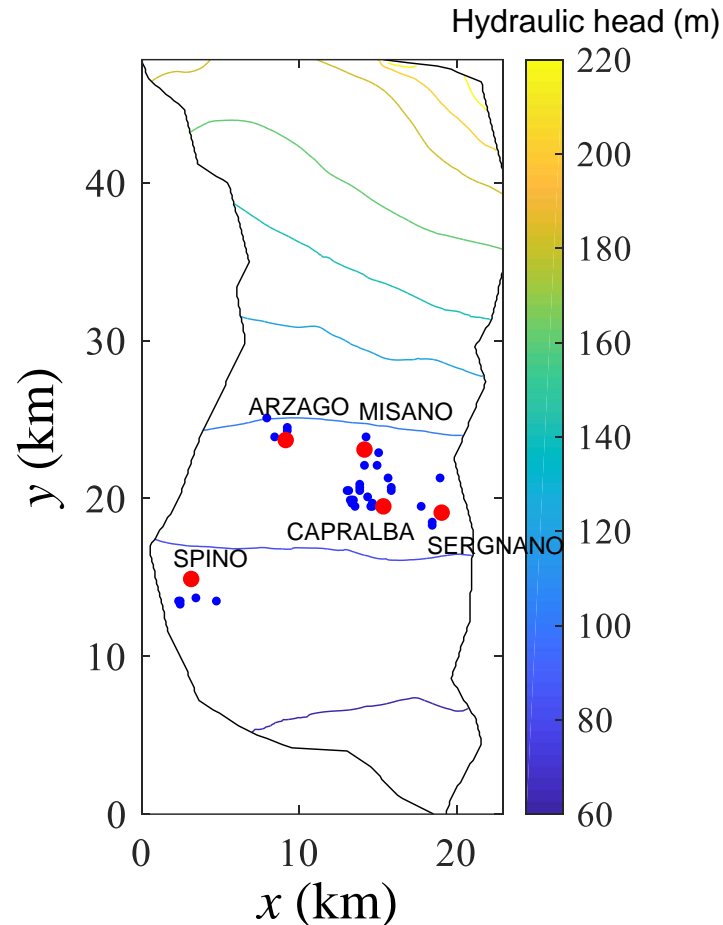


- What is the probability of spring depletion due to increasing exploitation of the aquifer?
- How does the uncertainty associated with the hydraulic conductivity affect the solution?



# Problem variables

- Springs ( $N_s = 34$ )
- Pumping wells with variable rates ( $N_w=5$ )



**Design variables:** pumping well rates at a subset of 5 selected pumping wells

$$\mathbf{Q} = [Q_1; Q_2; Q_3; Q_4; Q_5] \quad Q_i^{\min} - Q_i^{\max}$$

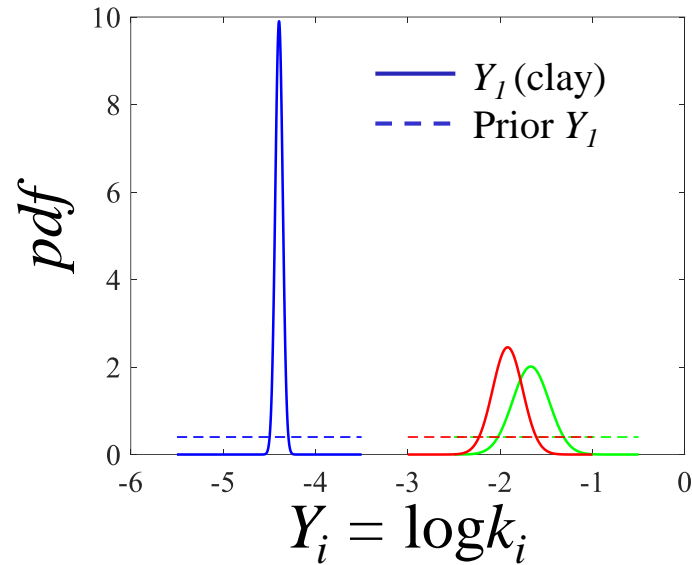
$$\Phi = \sum_{i=1}^{N_w} Q_i \quad \text{Function to be maximized: total sum of well rates}$$

**Failure event (constrain):** the hydraulic head at the spring cells is beyond a specific threshold

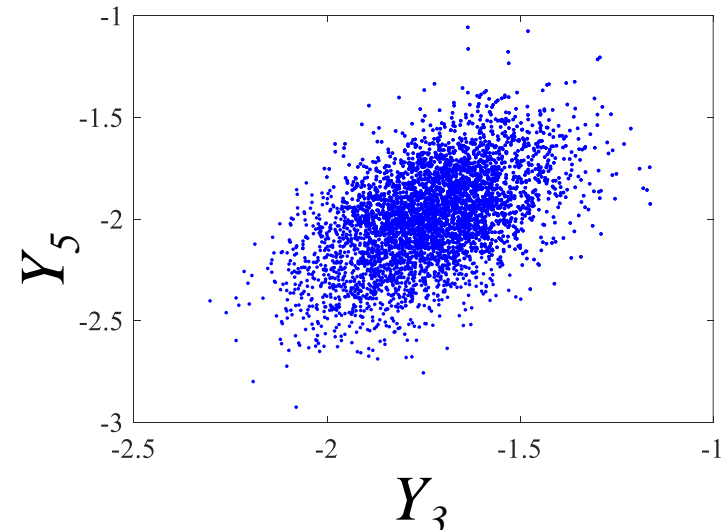
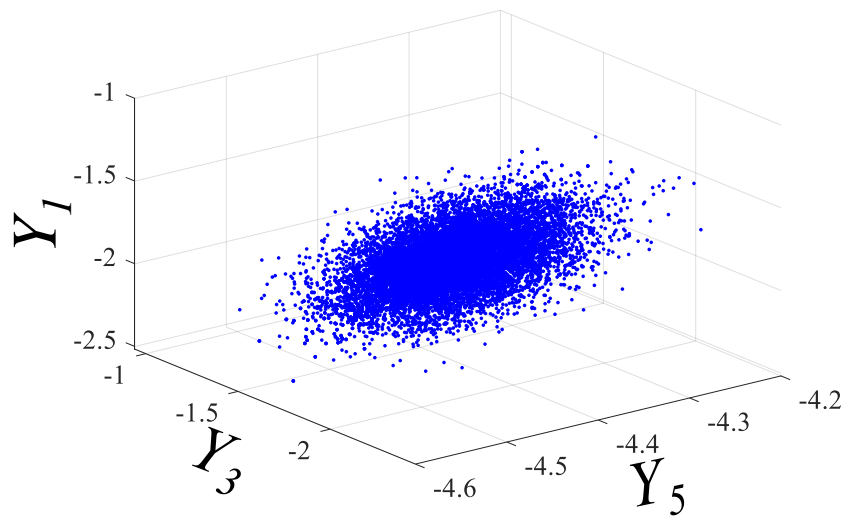
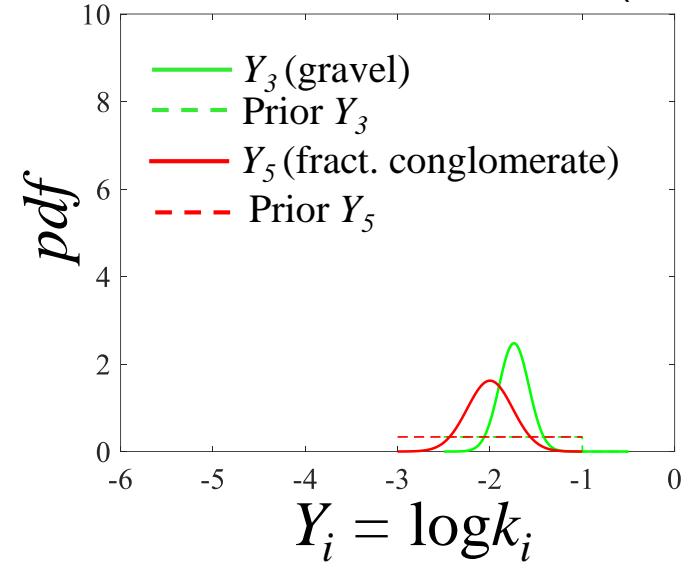
$$h_i(\mathbf{Q}, \mathbf{Y}) - h_i^{\min} \leq 0 \quad i = 1, N_s$$

# Uncertainty in log-conductivities

## COMPOSITE MEDIUM



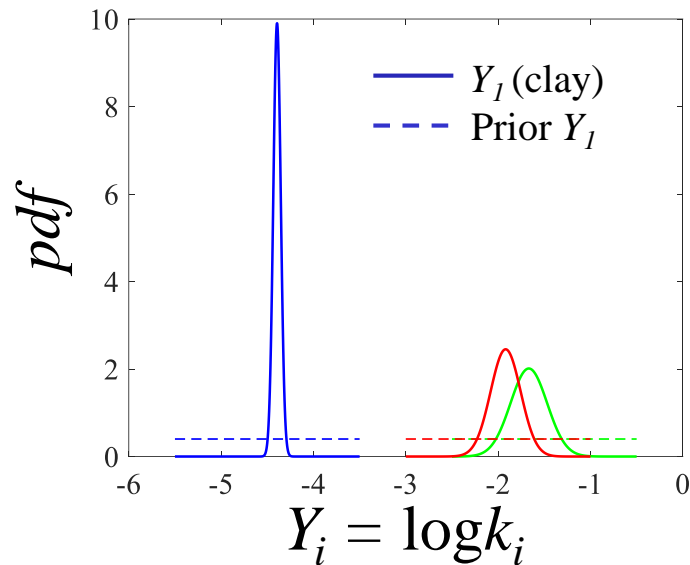
## OVERLAPPING CONTINUA (arithmetic)



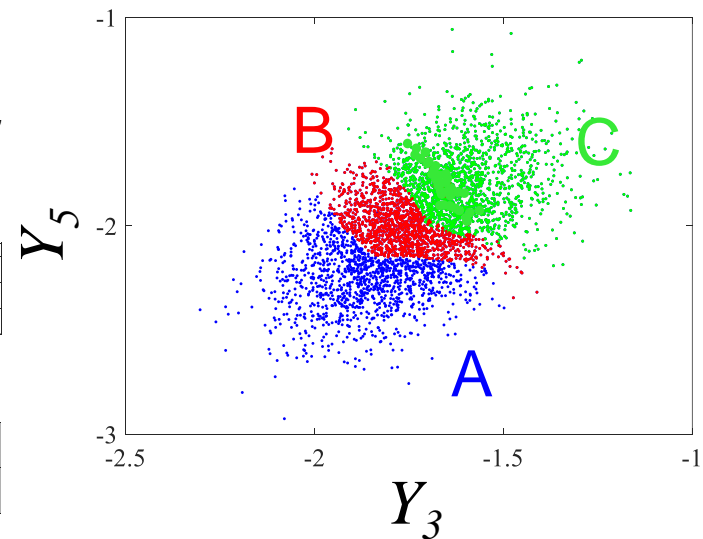
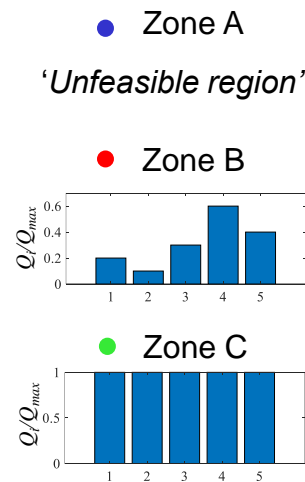
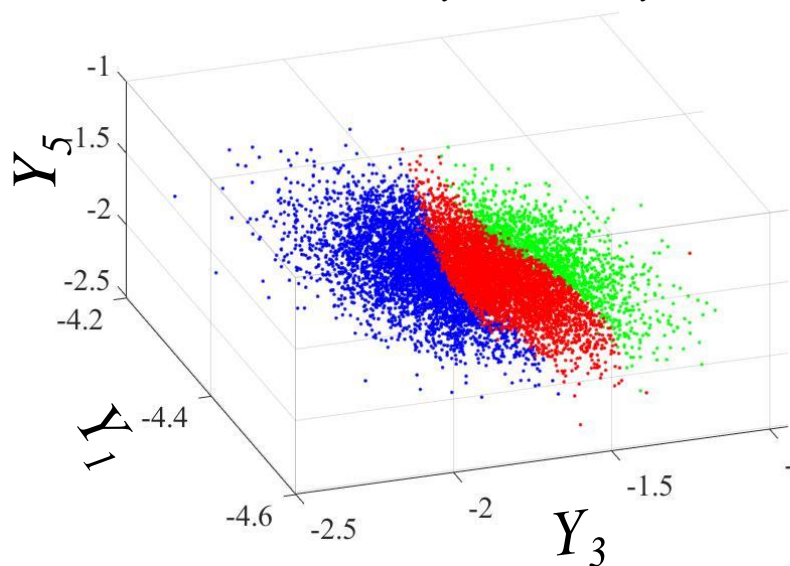
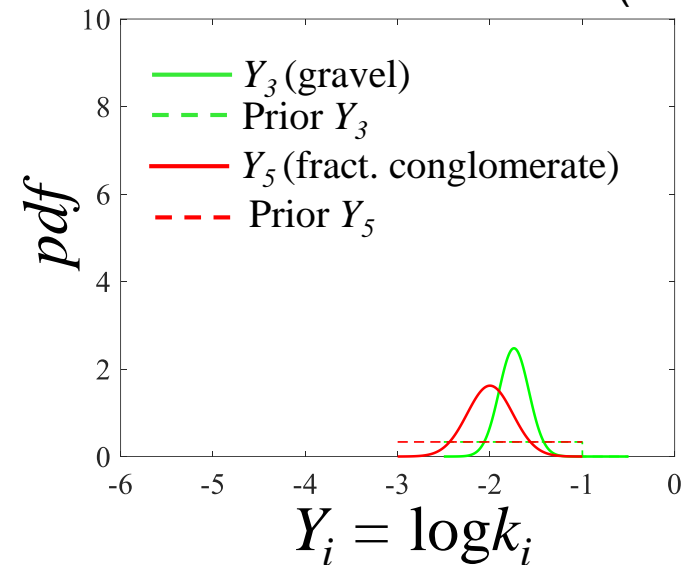


# Uncertainty in log-conductivities

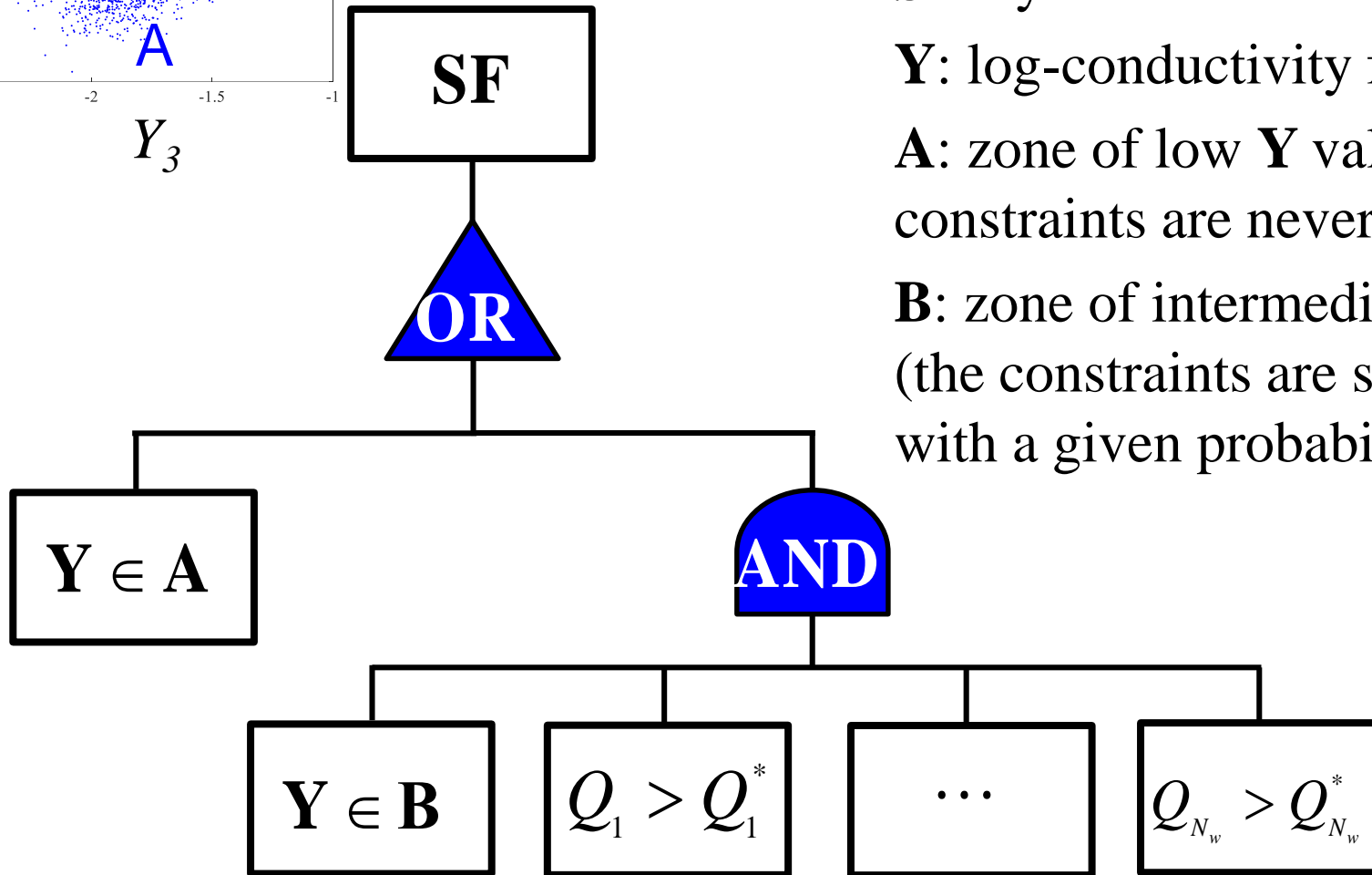
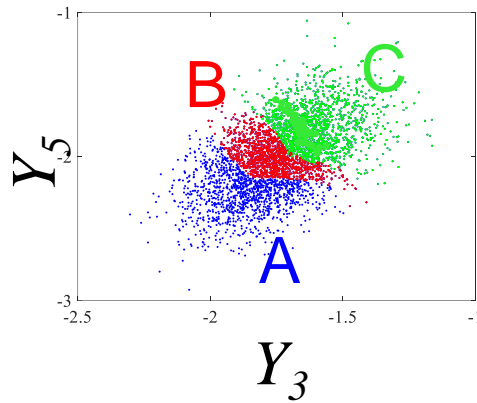
## COMPOSITE MEDIUM



## OVERLAPPING CONTINUA (arithmetic)



# Fault tree analysis



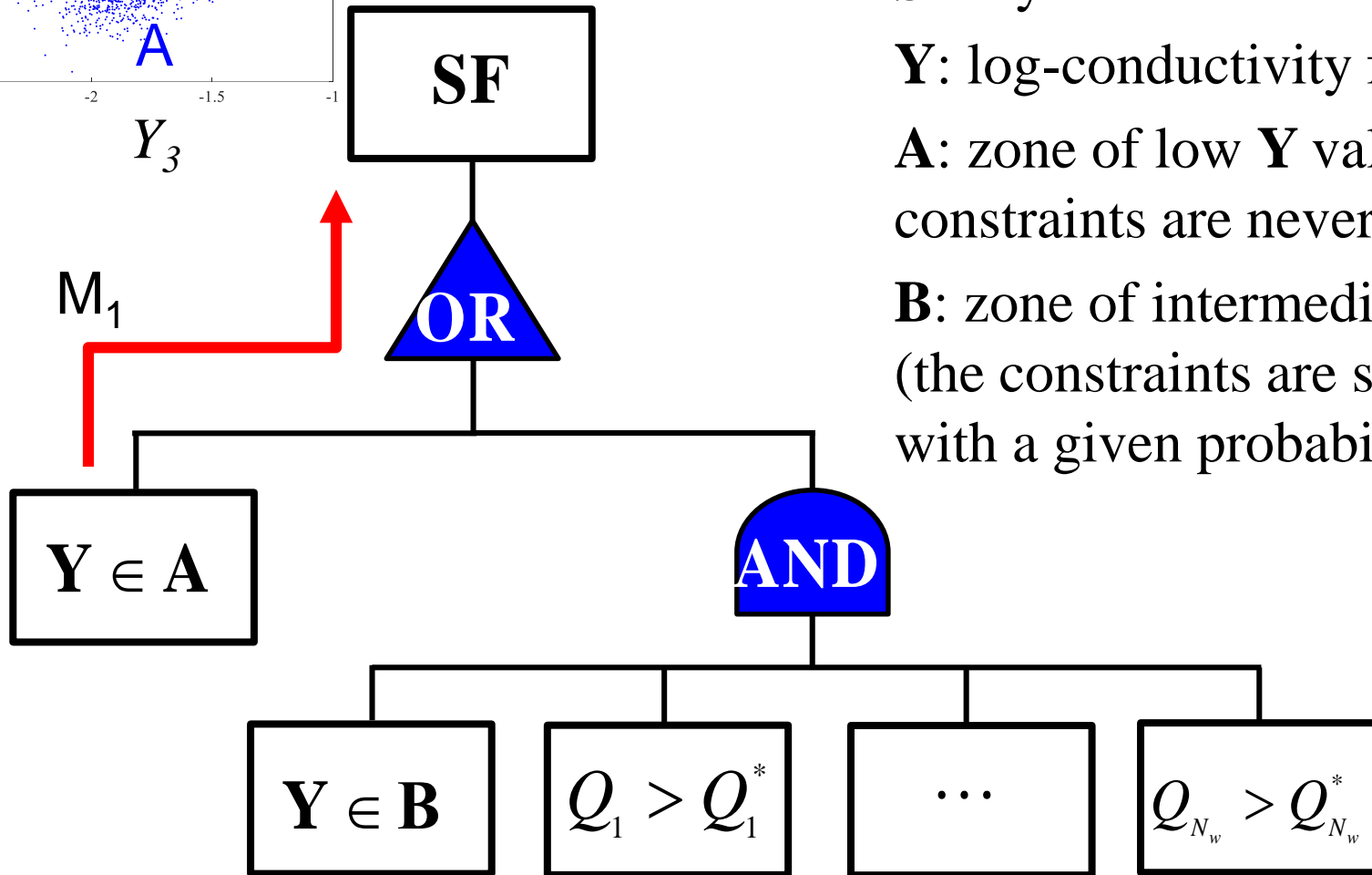
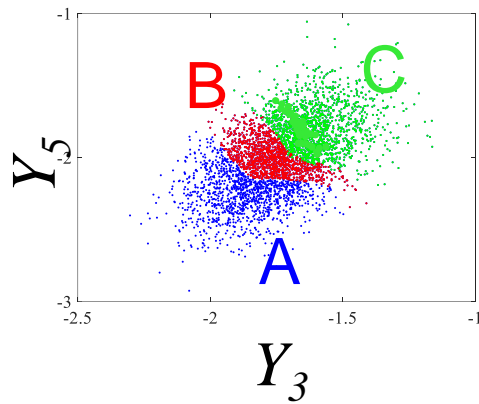
**SF**: system failure

**Y**: log-conductivity field

**A**: zone of low **Y** values (the constraints are never satisfied)

**B**: zone of intermediated **Y** (the constraints are satisfied with a given probability)

# Fault tree analysis



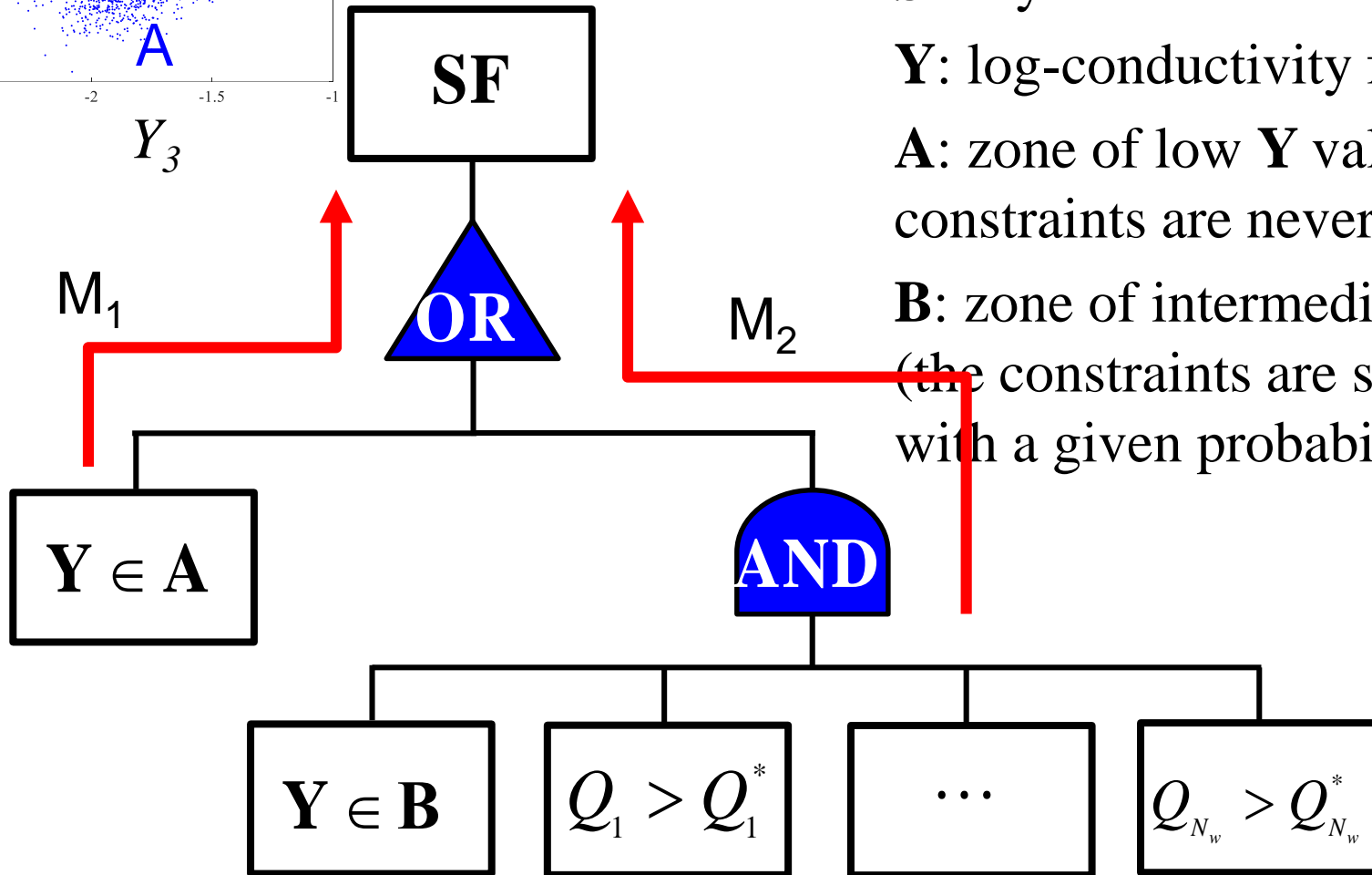
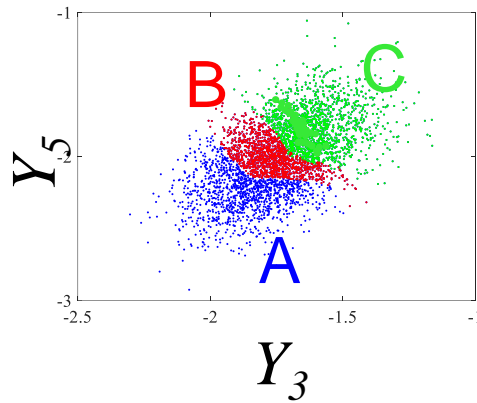
**SF**: system failure

**Y**: log-conductivity field

**A**: zone of low **Y** values (the constraints are never satisfied)

**B**: zone of intermediated **Y** (the constraints are satisfied with a given probability)

# Fault tree analysis



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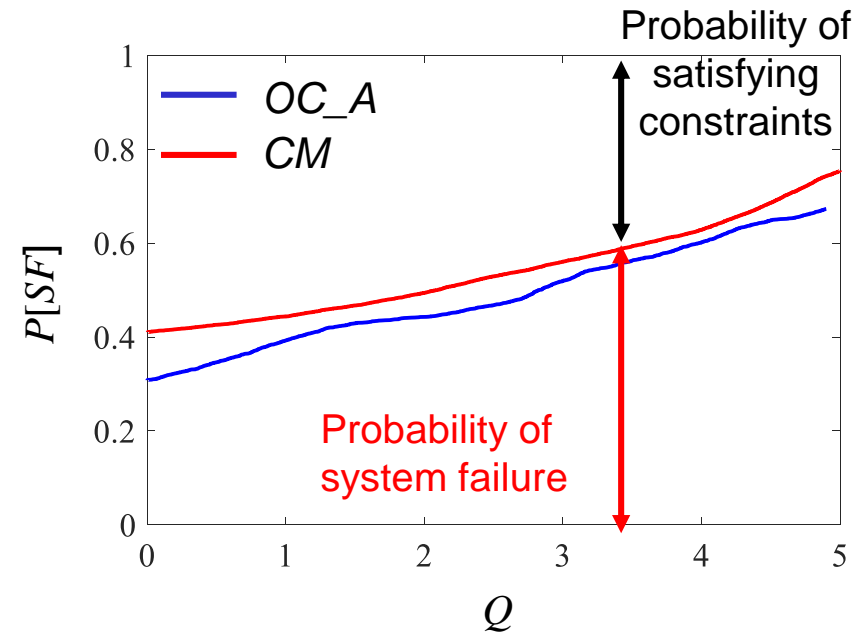
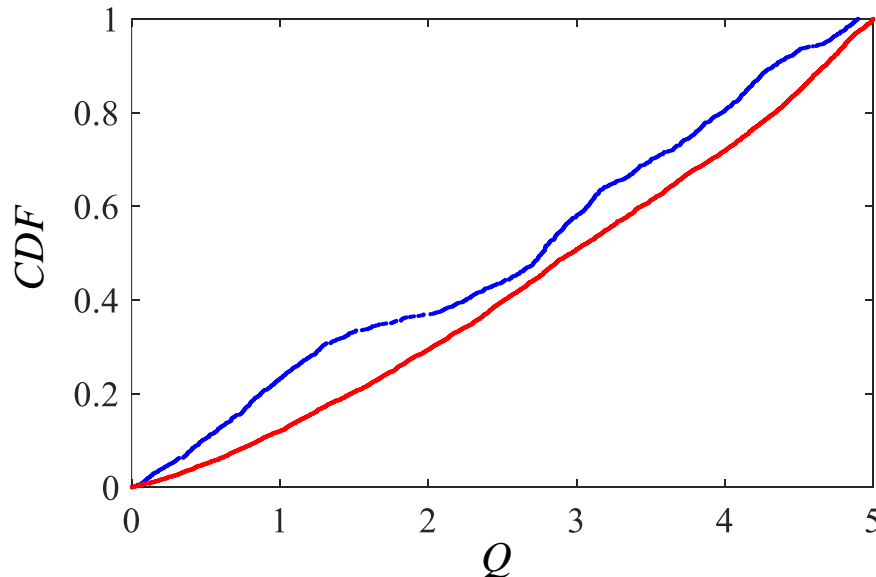
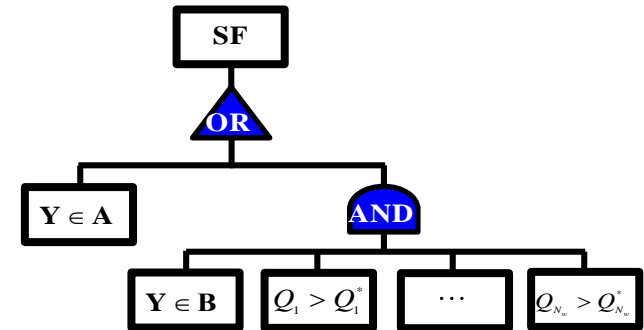
**B**: zone of intermediated **Y** (the constraints are satisfied with a given probability)

# Example of results

- Considering the sum of the 5 wells flow rates

$$Q = \sum_{i=1}^5 (Q_i / Q_i^{\max})$$

$$P[SF] = \frac{N_A}{N_{tot}} + CDF(Q) \frac{N_B}{N_{tot}}$$





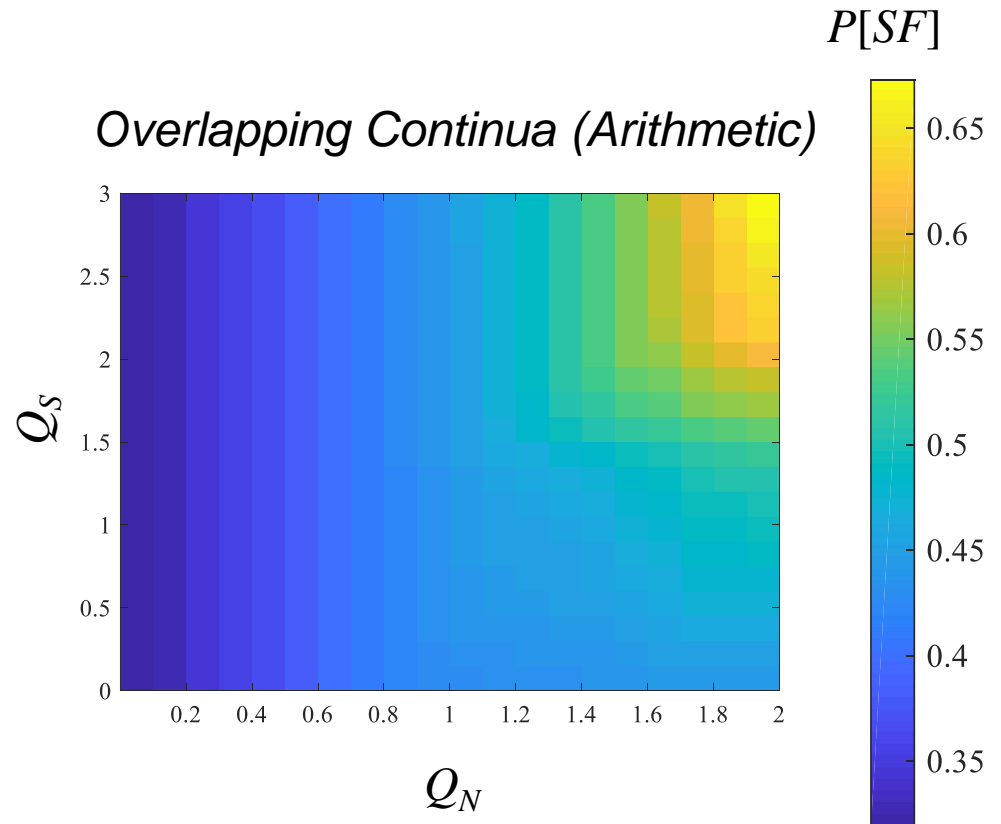
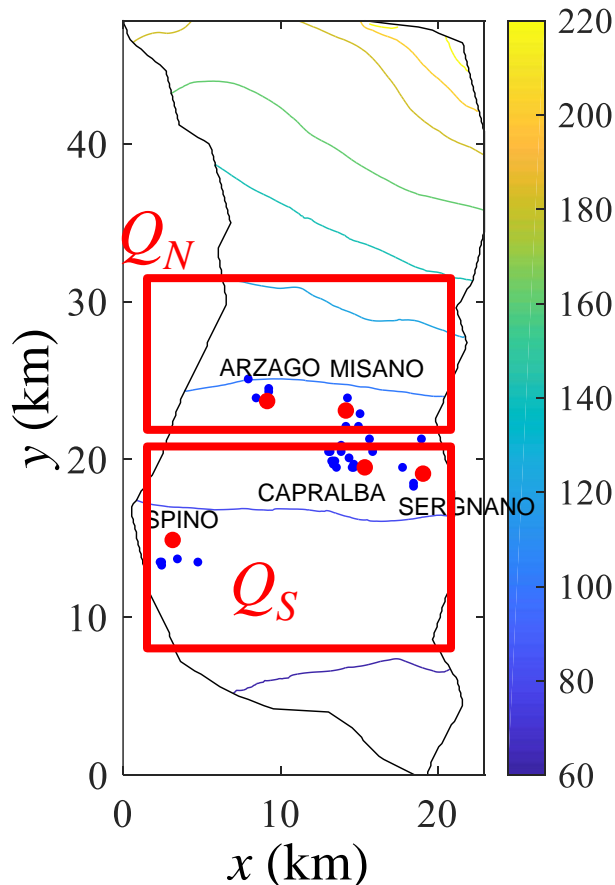
- Considering the Northern and Southern wells separately

$$Q_N = \sum_{i=1}^2 (Q_i / Q_i^{\max})$$

$$Q_S = \sum_{i=3}^5 (Q_i / Q_i^{\max})$$

$$P[SF](Q_N, Q_S) = \frac{N_A}{N_{tot}} + CDF_{\text{joint}}(Q_N, Q_S) \frac{N_B}{N_{tot}}$$

Hydraulic head (m)



- We compares a set of Global Sensitivity Analysis (GSA) approaches in the context of groundwater flow in a three-dimensional large scale groundwater system
- Albeit being based on differing metrics and concepts, the three GSA approaches analyzed lead to similar and consistent rankings of parameters which are influential to the target model outcomes
- GSA results provide insight about model calibration and model parameter reduction
- Risk assessment, based on a Fault Tree analysis, allows to identify optimal pumping rates preserving the springs' activity

# Natural spring' protection and groundwater management under uncertainty conditions: the Cremona Aquifer



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